63-71 WATERLOO ROAD, MACQUARIE PARK STORMWATER MANAGEMENT PLAN

DRAWINGS INDEX

DR-000	LEGEND
DR-100	CONCEPT STORMWATER LAYOUT - GROUND FLOOR
DR-110	STORMWATER DETAILS - SHEET 1
DR-120	CATCHMENT PLAN & OSD FLOWS
DR-130	DRAINS MODEL (5% AEP) - ROAD 1 + SITE
DR-131	DRAINS MODEL (1% AEP) - ROAD 1 + SITE
DR-140	ROAD 1 DRAINAGE LONG SECTIONS - SHEET 1
DR-141	ROAD 1 DRAINAGE LONG SECTIONS - SHEET 2
DR-200	TYPICAL PIT DETAILS - SHEET 1

NOTES:

- 1. ALL DIMENSIONS TO BE CONFIRMED ON SITE PRIOR TO CONSTRUCTION.
- 2. SITE LAYOUT BASED ON ARCHITECTURAL PLANS BY A+ DESIGN GROUP (DECEDMBER 2021) AND SURVEY PLANS BY LTS LOCKLEY REGISTERED SURVEYORS (25/05/2018).
- 3. LOCATION OF ALL SERVICES MUST BE CONFIRMED ON SITE PRIOR TO COMMENCEMENT OF EXCAVATION WORKS.
- 4. ALL STORMWATER DRAINAGE PIPES AND ASSOCIATED DEVICES ARE TO BE INSTALLED IN ACCORDANCE WITH THE RELEVANT STANDARDS, THE BUILDING CODE OF AUSTRALIA, MANUFACTURER'S RECOMMENDATIONS, SYDNEY CATCHMENT AUTHORITY RECOMMENDED PRACTICE, AND LOCAL COUNCIL, AS APPLICABLE.
- 5. ALL INVERT LEVELS PROVIDED ON THIS DRAWING ARE REDUCED TO AHD AND BASED ON INTERPOLATED SURFACE LEVELS AND SYSTEM REQUIREMENTS.

- 6. WHERE POSSIBLE, PIPEWORK SHALL BE LOCATED EXTERNAL TO THE BUILDING.
- 7. STORMWATER PIPES TO BE GRADED AT A MINIMUM 1% UNLESS NOTED OTHERWISE.
- 8. ALL PIPE AND CONDUITS TO BE MARKED IN ACCORDANCE WITH AS1345 - 1995.
- 9. TRENCHES AND SERVICE SEPARATIONS IN ACCORDANCE WITH AS/NZS 5601, AS/NZS 3500, AND AS/CA S009.

LEGEND:



DRAINAGE PIT & SIZE



INSPECTION OPENING

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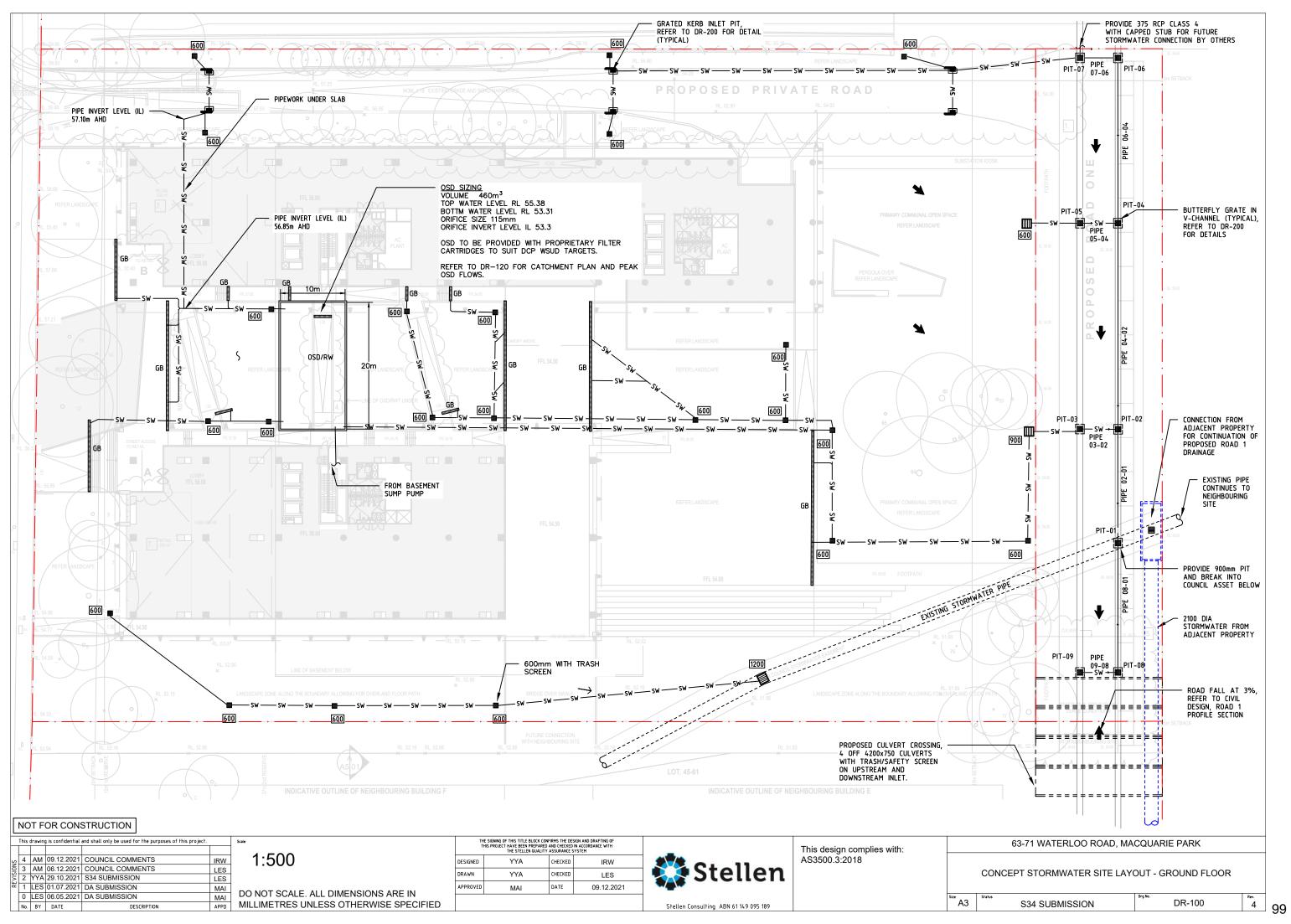
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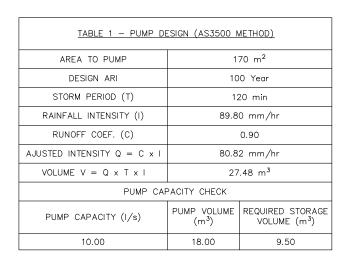
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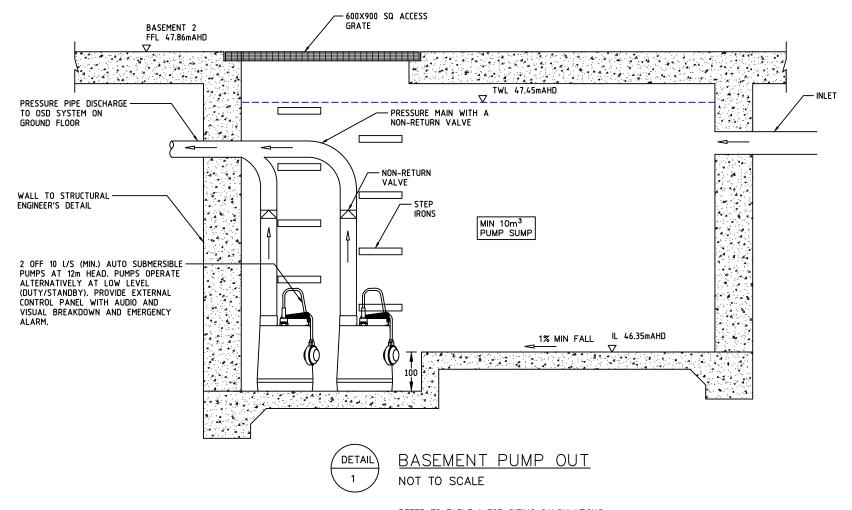
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Stellen Consulting ABN 61149 095 189

This design complies with: AS3500.3:2018

		63-71 WATERLOO ROAD	, MACQUAF	RIE PARK	
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Size A3	Status	S34 SUBMISSION	Prg ND.	DR-000	Rev. 4







REFER TO TABLE 1 FOR SIZING CALCULATIONS

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This design complies with:
AS3500.3:2018

63-71 WATERLOO ROAD, MACQUARIE PARK

STORMWATER DETAILS - SHEET 1

Size A3 Status S34 SUBMISSION DR-110

100

NOTE:
THE PROPOSED DEVELOPMENT IS AFFECTED BY FLOODING DURING THE 1% AEP EVENT TO APPROXIMATELY 52.95 mAHD. THE GROUND FLOOR LEVEL OF THE PROPOSED DEVELOPMENT VARIES THROUGHOUT THE SITE BUT IS GENERALLY

AS A RESULT OF THE SITE LEVELS IT IS NOT POSSIBLE TO PROVIDE ON-SITE DETENTION OR STORMWATER QUALITY TREATMENT MEASURES AT OR BELOW THE GROUND FLOOR LEVEL WITHOUT BEING AFFECTED BY FLOODING. ANY UNDERGROUND OSD PROVIDED AT THIS LEVEL IS SUSCEPTIBLE TO FLOODING AND TAILWATER EFFECTS DURING THE 1% AEP AND WOULD BE INEFFECTIVE. TOTAL FLOOD AFFECTED APEA 23/488 m² FLOOD AFFECTED AREA ≈ 3488 m²

INSTEAD, OSD AND WSUD MEASURES HAVE BEEN PROVIDED WITHIN BELOW THE GROUND FLOOR LEVEL OF THE BUILDING AND ABOVE THE PREDICTED 1% AEP FLOOD LEVELS.

FOR THE PURPOSE OF CALCULATING OSD VOLUMES AND ASSESSING THE EFFICACY OF THE PROPOSED WSUD MEASURES THOSE AREAS DIRECTLY AND INDIRECTLY AFFECTED BY FLOODING HAVE BEEN EXCLUDED FROM THE

IN ADDITION TO THE ABOVE, THE PROPOSED CONTINUATION OF ROAD 01 (TOTAL AREA \approx 2220 m^2) has not been included in any osd/wsud calculations. Ultimately the road will be handed over to council and is not considered to be part of the site.

BASED ON THE ABOVE, THE FOLLOWING "SITE AREAS" HAVE BEEN ADOPTED:

14053 m² 5763 m² (41%) 8290 m² (59%) TOTAL SITE ARE **PERVIOUS** IMPERVIOUS AREA TO OSD

7202 m² 1198 m² (16.6%) PERVIOUS 6004 m² (83.4%) **IMPERVIOUS**

AREA BYPASS OSD

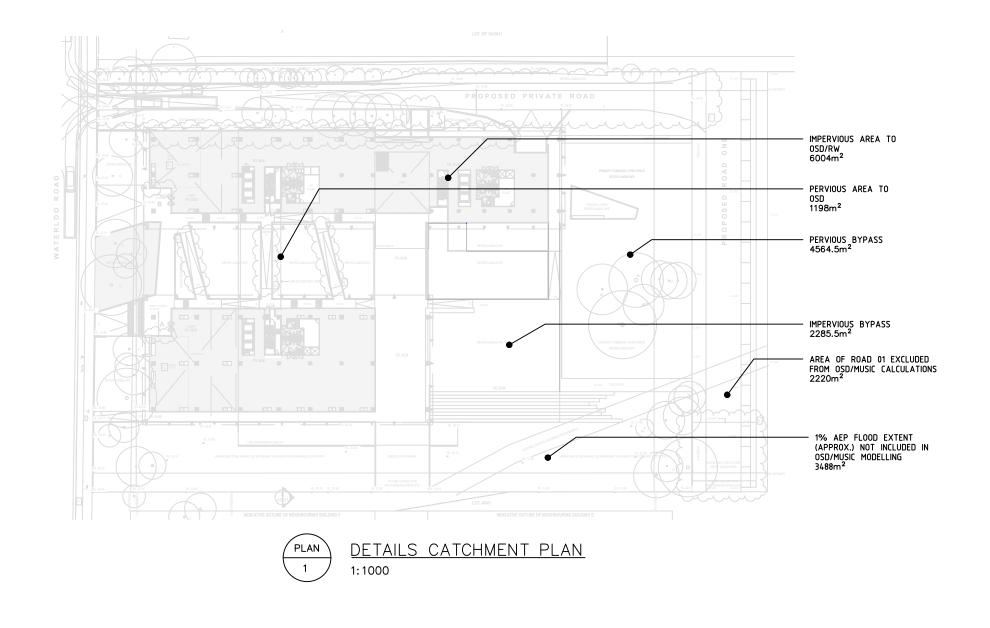
6851 m² 4565.5 m² (66.6%) 2285.5 m² (33.4%) PERVIOUS **IMPERVIOUS**

OSD CALCULATIONS AND NOTES:

- 1. OSD MODELLED USING "DRAINS" BY WATERCOM
- 2. PSD CALCULATED FROM 5YR PEAK POST-DEVELOPMENT FLOW IN ACCORDANCE WITH COUNCIL DCP REQUIREMENTS.

TABLE 1 - SITE OSD PEAK FLOWS

Event	PSD (I/s)	OSD (I/s)	Bypass (I/s)	OSD + Bypass ≤ PSD
0.2 EY	413	25	186	Yes
5% AEP	413	31	285	Yes
1% AEP	413	37	375	Yes



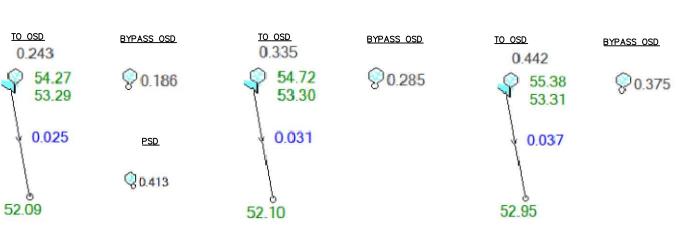


FIGURE 1 - 0.2EY OSD MODEL (DRAINS) FIGURE 1 - 5% AEP OSD MODEL (DRAINS) FIGURE 3 - 1% AEP OSD MODEL (DRAINS)

AREA TO OSD/RW 7202 m² AREA BYPASS OS/RW 6851 m² ROAD 2220 п AREA AFFECTED BY FLOODING 3488 m² PLAN KEY CATCHMENT PLAN

2 1:2000

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		63-71 WATERLOO ROAD, MA	ACQUARIE PARK	
		CATCHMENT PLAN AND	OSD FLOWS	
*A3	Status	S34 SUBMISSION	DR-120	Rev.

MODELING NOTES:

- 1. ROAD DRAINAGE DESIGN COMPLETED USING DRAINS HYDRAULIC MODELING SOFTWARE.
- 2. COUNCIL PROVIDED DRAINS MODELING RESULTS USED AS UPSTREAM INPUTS FOR PIPED AND OVERLAND FLOW

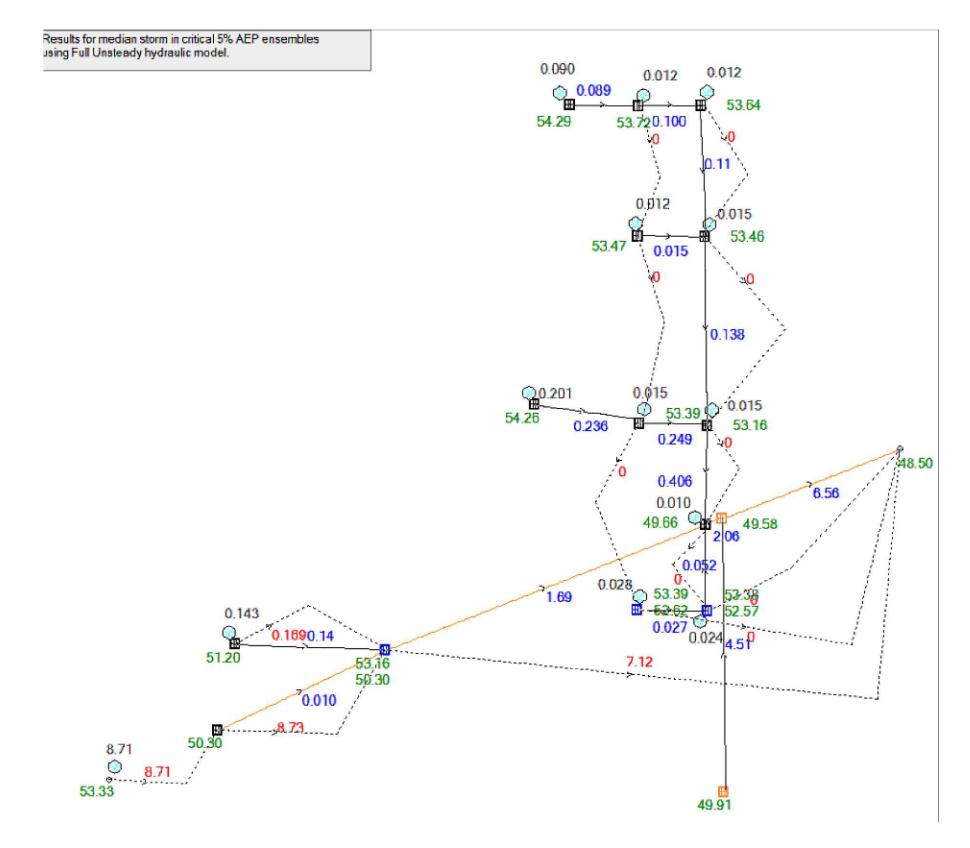


FIGURE 3 - 5% AEP ROAD 1 DRAINAGE MODEL (DRAINS)

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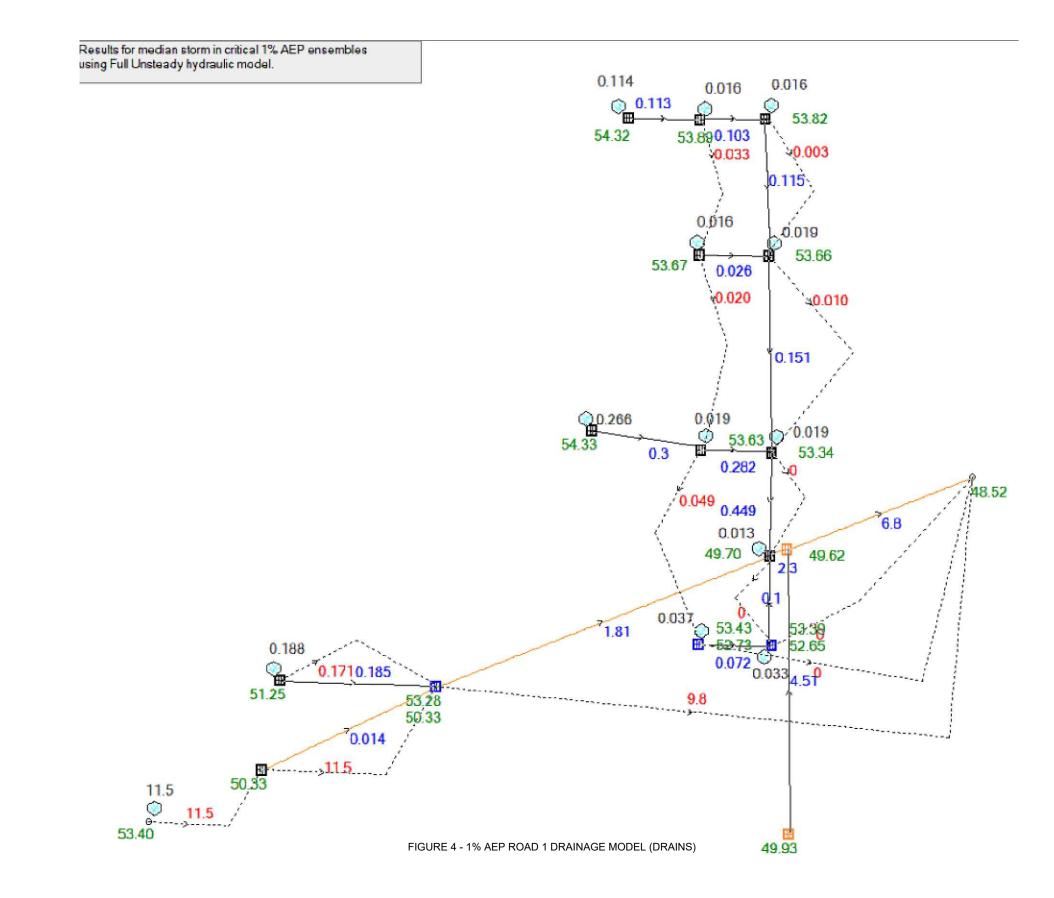
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AS3500.3:2018			DDAING MODEL (50) AED)	DO 4 D 4	. 0
			DRAINS MODEL (5% AEP) - I	ROAD 1	+ SIIE
	Size A3	Status	S34 SUBMISSION	Drg No.	DR-130

MODELING NOTES:

- 1. ROAD DRAINAGE DESIGN COMPLETED USING DRAINS HYDRAULIC MODELING SOFTWARE.
- 2. COUNCIL PROVIDED DRAINS MODELING RESULTS USED AS UPSTREAM INPUTS FOR PIPED AND OVERLAND FLOW



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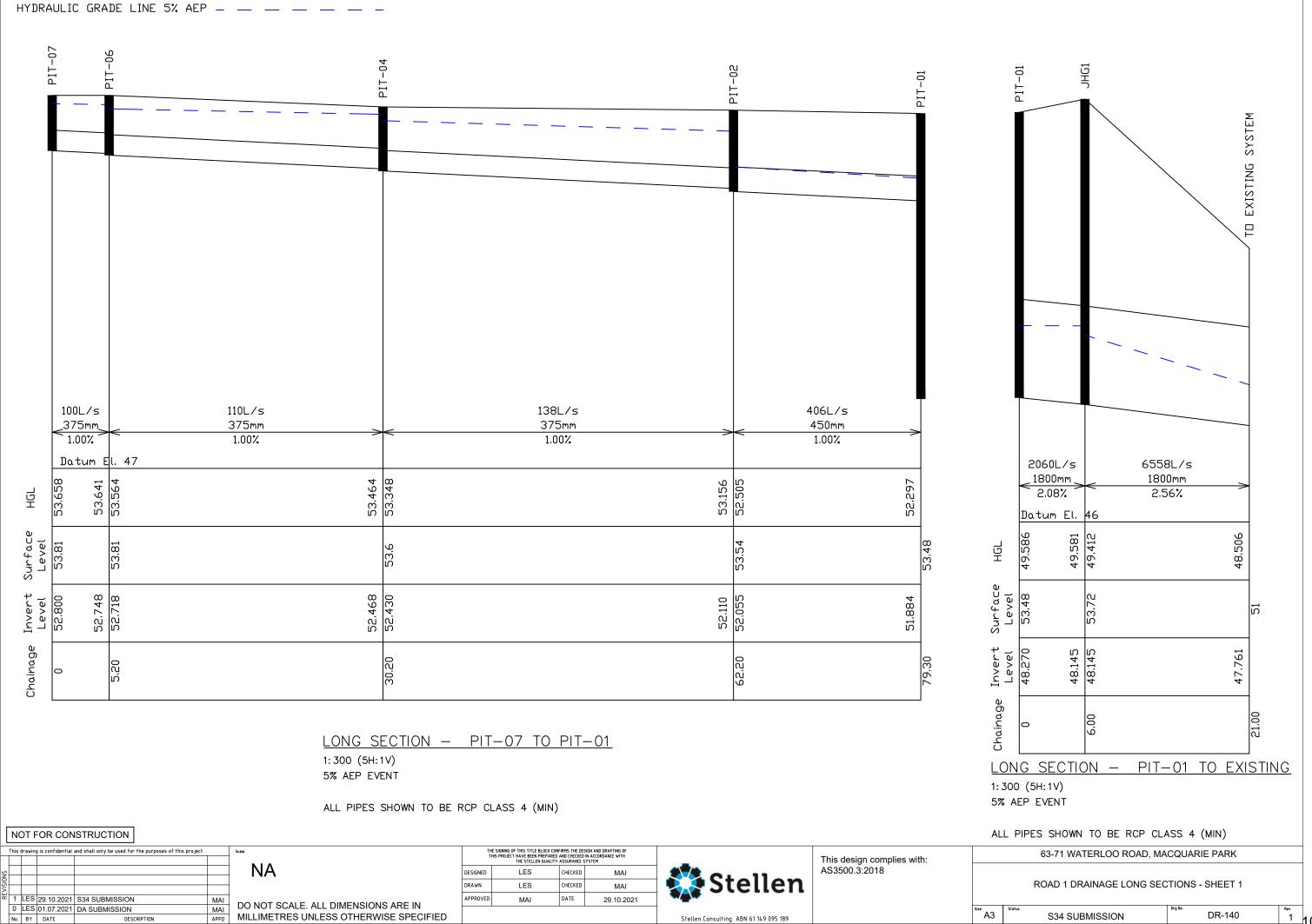
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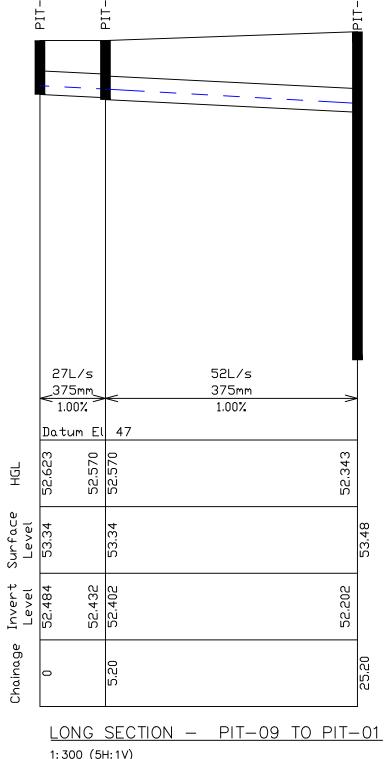
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This design complies with:			63-71 WATERLOO ROAD, MA	CQUARI	E PARK
AS3500.3:2018			DRAINS MODEL (1% AEP) -	ROAD 1	+ SITE
			, ,		
	Size A3	Status	S34 SUBMISSION	Drg No.	DR-131

103



No. BY DATE



1: 300 (5H:1V) 5% AEP EVENT

ALL PIPES SHOWN TO BE RCP CLASS 4 (MIN)

 Chainage Invert Surface
 HGL

 Level Level
 52.824
 53.465
 Phone Pit-05

 52.824
 53.465
 Phone Pit-05

 52.772
 53.464
 Phone Pit-05

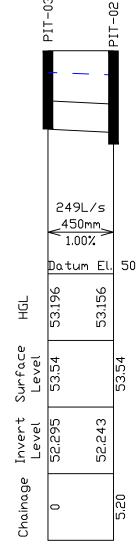
 5.20
 53.464
 Phone Pit-05

 5.20
 53.6
 Chain Pit-05

LONG SECTION - PIT-05 TO PIT-04

1:300 (5H:1V) 5% AEP EVENT

ALL PIPES SHOWN TO BE RCP CLASS 4 (MIN)



LONG SECTION - PIT-03 TO PIT-02

1:300 (5H:1V) 5% AEP EVENT

ALL PIPES SHOWN TO BE RCP CLASS 4 (MIN)

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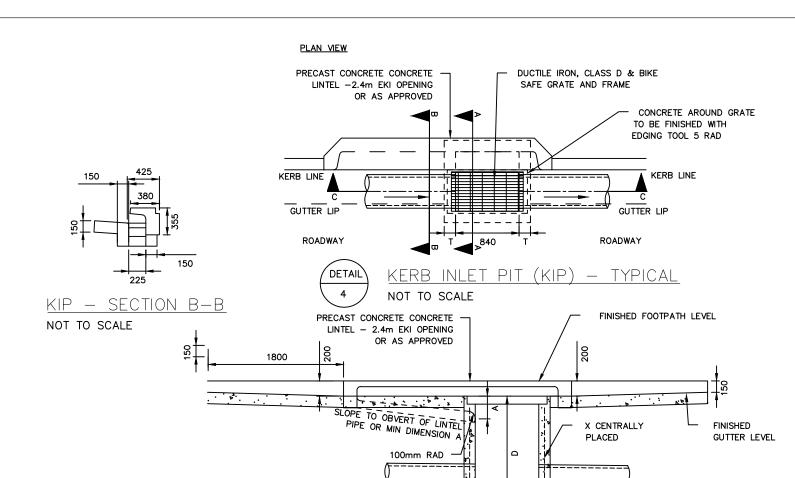
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	63-71 WATERLOO ROAD, MACQUARIE PARK						
ROAD 1 DRAINAGE LONG SECTIONS - SHEET 2							
	Size A3	Status	S34 SUBMISSION	Drg No. DR-141	Rev.		



CORNER BARS TYP

Y CENTRALLY PLACED

KIP - SECTION C-C

DIT OR			
<u>PIT_SPECIFICATION</u>			
DEPTH (D)	750-1200	1200-2100	
BASE THICKNESS (B)	150	200	
SIDE THICKNESS (T)	150	180	
VERTICAL MESH (X)	SL82	SL82	
HORIZONTAL MESH (Y)	SL81	SL81	
CORNER BARS	N12-300 L 500	N12-200 L 600	

NOT TO SCALE PRECAST CONCRETE LINTEL - 2.4m EKI OPENING DUCTILE IRON, CLASS D & OR AS APPROVED BIKE SAFE GRATE AND FRAME FOOTPATH AS PER STANDARD X CENTRALLY PLACED 75 CORNER BARS TYP 75mm MINIMUM BENCHING TO Y CENTRALLY HALF PIPE HEIGHT TOTAL BENCHING TO OBVERT OF PIPE PLACED

KIP - SECTION A-A

NOT TO SCALE

840

75mm MINIMUM BENCHING TO

HALF PIPE HEIGHT TOTAL BENCHING TO OBVERT OF PIPE

NOTES:

- 1) COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS TO BE AT 32MPa 75mm MINIMUM BENCHING TO HALF HEIGHT & TOTAL BENCHING TO OBVERT OF
- 3) DN100 SUBSOIL DRAINAGE PIPE 3.0M LONG WRAPPED IN FABRIC SOCK TO BE PROVIDED IN PIPE TRENCHES ADJANCT TO INLET PIPES
 4) PROVIDE STEP IRONS WHERE PIT IS DEEPER THAN 1m AT 300mm CENTRES
- 5) PITS OVER 2.1m IN DEPTH TO BE DESIGNED BY STRUCTRAL ENGINEER.
- 6) GRATES SHALE BE BICYCLE SAFE AND HAVE MAXIMUM INLET CAPACITY. ALL GRATES MUST BE APPROVED BY COUNCIL
- 7) REINFORCMENT TO COMPLY WITH AS1302, AS1303 & AS1304

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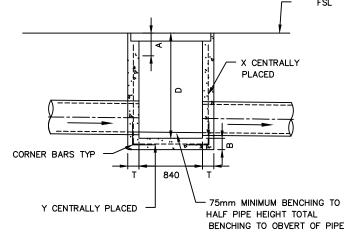
GUTTER LIP GUTTER LIF 840 ROADWAY ROADWAY DETAIL GRATED SURFACE INLET PIT IN SWALE - TYPICAL 5 NOT TO SCALE FSL

CONCRETE AROUND GRATE

TO BE FINISHED WITH

EDGING TOOL 5 RAD

KERB LINE



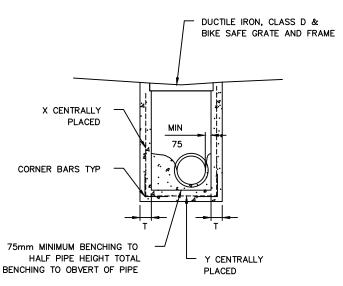
DUCTILE IRON, CLASS D & BIKE -

V-CHANNEL EDGE -

KERB LINE

SAFE GRATE AND FRAME

GSIP IN DRIVEWAY - SECTION E-E NOT TO SCALE



GSIP IN SWALE - SECTION D-D NOT TO SCALE

This design complies with:			63-71 WATERLOO ROAD, MA	ACQUARIE PARK	
AS3500.3:2018					
			TYPICAL PIT DETAILS	- SHEET 1	
	Size A3	Status	S34 SUBMISSION	DR-200	Rev.

106